

Model for Estimating the Time of Concentration in Watersheds

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Abstract The time of concentration for a watershed is an essential parameter to the design of any hydrological project. In this study, a time of concentration estimation-model that uses variables obtained by monitoring rainfall-runoff events in a rural watershed of a tropical climate is proposed. In developing the model, the relationship between the time of concentration and independent variables was verified using a linear correlation matrix. Two variables with the highest correlation coefficient were selected to derive a model that can estimate the time of concentration. The Harmony Search (HS) optimization algorithm was employed to optimize the model parameters. The performance of the model was evaluated based on the Nash–Sutcliffe (NS) coefficient, which yielded a value greater than 0.80. The proposed model allows one to estimate the time of concentration using only the hydrograph of an event at the base level of a watershed, without the use of rainfall data.

Keywords Hydrographs · Surface runoff · Time factors · Hydrology · Parameters · Harmony search

1 Introduction

The time of concentration concept was first explored by Mulvaney (1851) to describe the effects of rain on watersheds. The same author, in considering uniform rainfall over a drainage area, studied the time at which runoff reaches a maximum level, i.e., the time required for an entire

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watershed area to contribute to flow at the outlet point. The term was officially coined by Kuichling (1889), who studied rainfall-runoff relationships in populous neighborhoods of Rochester, New York (USA).

Several researchers (Simas and Hawkins 2002; Ogbonna 2004; and Li and Chibber 2008, among others) have derived eqs. to estimate the time of concentration. Each equation, derived empirically or analytically, emerged from studies conducted in a specific discipline and was modified to consider physical and hydrological characteristics of a given location. These eqs. have been frequently applied in the works of Kang et al. (2008); Upegui and Gutiérrez (2011), and Liang and Melching (2012), which included estimates of the time of concentration. Studies by Fang et al. (2008) and Sharifi and Hosseini (2011), among others, have identified variations in results obtained from various empirical equations. Fang et al. (2008) found wide differences between times of concentration estimated by different formulas that use watershed parameters. These average relative differences range from -38 to 207 %. According to Sharifi and Hosseini (2011), the time of concentration is an immeasurable variable, and designers determine it by use of formulas, which stem from varied surrogates and yield different results.

Definitions of the time of concentration have grown more sophisticated over time, according to a review of the perspectives considered. Eagleson (1970) defines it as the time required for surface runoff to reach a state of equilibrium. According to Chow et al. (1988), the time of concentration corresponds to the time required for an effective or excess raindrop to travel from the farthest point of a watershed to its outlet point. McCuen et al. (1984) refer to it as the time required for a drop of water to reach the farthest point of the watershed surface area (in the hydraulic path) before reaching the outlet point. The same author, who conducted studies using data for American urban watersheds, defines the time of concentration as the time elapsed from the end of effective rainfall to the end of the surface runoff. Su and Fang (2004) describe the time of concentration as the period running from the start of effective rainfall to the point at which flow reaches 98 % of equilibrium runoff. Wong (2005) defines it as the time elapsed since the start of effective rainfall to the time at which 95 % of the equilibrium runoff occurs. In this study the authors define the time of concentration as the time interval from the end of the rainfall period to the end of the surface runoff period.

Understanding watershed behaviors relating to the time of concentration allows for the prevention and mitigation of consequences of natural disaster and point source pollution in water resources. The changes in land use associated with urban development, replacing permeable soil by impervious surfaces, affect runoff. Gwenzi and Nyamadzawo (2014) cite that urbanization alone reduces the time of concentration, and increases peak discharge, and runoff volumes and velocity for the on-site and downstream hydrographs. According to Konrad (2003), with less storage capacity for water in urban basins and more rapid runoff, urban streams rise more quickly during storms and have higher peak discharge rates than do rural streams. Thus, the time of concentration in rural watersheds tends to be larger than time of concentration in urban watersheds.

According to Wong (2009), among all watershed response time parameters, the time of concentration is the most widely used parameter. Bondelid et al. (1982) showed that as much as 75 % of the total error in an estimate of the peak discharge can result from errors in the time of concentration. Recognizing the importance of the time parameters in hydrological design and water resource management, many studies have addressed the problem of estimating the time of concentration. Furthermore, the time of concentration reflects important physiographic features of a watershed. Pavlovic and Moglen (2008) argue that this parameter reflects the speed at which a drained area responds to rainfall events. Fang et al. (2008) note that if the time

of concentration is underestimated, peak flow is overestimated and vice versa. The authors thus, stress the importance of accurate time of concentration estimations.

The majority of studies that have focused on the development of relevant equations to time of concentration have been based on data for watersheds of temperate regions. However, time of concentration and runoff generation processes are related to watershed responses to rainfall events. According to Dykes and Thornes (2000), as tropical soils exhibit varying characteristics, experiments that examine mechanisms of runoff generation in tropical and subtropical climates are necessary, as these mechanisms may differ under various conditions. This study thus formulates a methodology for estimating the time of concentration in rural watersheds of tropical climates using variables obtained through the monitoring of rainfall-runoff events. The sub-watershed used in this study is located in the capital city of Mato Grosso do Sul State, in the Brazilian Cerrado, and it is responsible for the most part of the urban water supply.

2 Materials and Methods

2.1 Study Area

The variables used in this study were obtained by monitoring rainfall-runoff events in the sub-watershed of the Guarairoba stream, near the City of Campo Grande, Mato Grosso do Sul State, Brazil [parallels 20° 28' and 20° 43' (south latitude) and meridians 54° 29' and 54° 11' (west longitude)], covering an area of 360 km². This sub-watershed is tributary of Botas River, which is contributing Pardo River, Paraná River affluent. The local valley bottoms characterized by extensive occurrence of wet fields, veredas and other typical gallery forests of Cerrado.

The pedology of the drainage area is composed of 94.1 % orthic Quartzarenic Neosol (RQo), 3.5 % hydromorphic Quartzarenic Neosol (RQg) (both derived from sandstone rock) and 2.4 % dystrophic Red Latosol (DRL) of basalt origin. The study accounted for Hydrologic Response Unit 2 (HRU 2) within a 158 km² area with a base level located in the middle course of the sub-watershed (Latitude: 20°34'43,417" S and Longitude: 54°19'58,335" W), having an elevation between 480 m and 660 m. Rainfall time series were obtained from data recorded at two rainfall, located at similar elevation, stations that operate at sites 1 and 4 (Fig. 1). Furthermore, we used double mass analysis as consistency tool of rainfall data and it was seen the homogeneity of the stations 1 and 4.

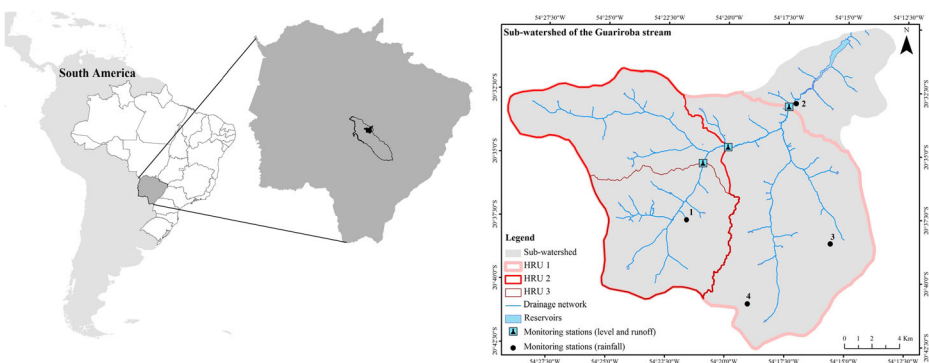


Fig. 1 Sub-watershed of the Guarairoba stream, its HRUs and the locations of measurement stations

The climate of the region, according to the Köppen climate classification model, is of the Aw type, defined as a tropical, hot and humid climate. This climate is characterized by temperatures that exceed 18 ° C during the winter season (June to August). It features well-defined rainy periods (November to April) falling within the range of 750–2.000 mm and pronounced drought in the months of July to September. The annual average rainfall for this region is 1.500 mm.

2.2 Hydrological Data

Watercourse level and rainfall depth data were continuously recorded, at an interval of 5 min, with the use of a datalogger throughout the years of 2011 to 2014. The discharge measurements were made, at varying intervals depending on the timing of rainfall periods observations, at the HRU 2 outlet. Hydrological data were post-processed in a spreadsheet to study the events.

2.3 Determination of Variables

The runoff data were prepared by applying the rating curve to the water-level data measured continuously. A continuous series of rainfall and runoff events has been analyzed, out of which of 17 distinct rainfall events representing the most possible variations of intensities and durations has been selected. The rainfall-runoff events selected in this study were all independent ones, unaffected by any other previous events.

The time of concentration (T_c) and other variables required for the study (below defined) were estimated via a graphical analysis of hyetographs and hydrographs for the selected events. No universally accepted graphical method has yet been developed for measuring the final point of effective rainfall. Thus, the T_c was defined as the time interval from the end of the rainfall period to the end of the surface runoff period.

In this study, reference to runoff means storm water runoff or the runoff occurring during and immediately following a rainfall event. According to Haan et al. (1994), the volume of storm water runoff is equal to the volume of rainfall excess or effective precipitation. Thus, the area below the runoff curve, which is bounded by the line that connects the start and end points of surface runoff, corresponds to the runoff volume. The start point is indicated by the beginning of the rise in the water-level. The end point is initially determined by the probable range of the end of the direct surface runoff. This range is evidenced by a tendency of a decreasing line segment of the graph in semi-logarithmic scale. Dividing the runoff volume by the study area generates the height of effective rainfall (h_{ef}). Using the total height of accumulated rainfall for each event (h_t) and the height of effective rainfall, the percentage of abstractions (P_{abs}) can be obtained, as shown in Eq. 1.

$$P_{abs} = \frac{(h_t - h_{ef})}{h_t} \cdot 100 \quad (1)$$

Other variables obtained from the graphical analysis of the events were defined as follows: the peak runoff (Q_p) is the maximum runoff level recorded for each event; rain duration (T_d) is the total duration of each rainfall period that gave rise to each event; average rainfall intensity (i_{avg}) is the total accumulated rainfall level divided by the total duration of the rainfall period; maximum intensity (i_{max}) is the maximum intensity level recorded in mm/5 min; (T_{CG}) is the elapsed time from the start to the rainfall centroid; (T_r) denotes the time interval from the start of the maximum runoff period to the end of the surface runoff period; (T_b) denotes the time elapsed between the start and end of the surface runoff period; (T_p) is the time interval between

the rainfall centroid and the start of the maximum runoff period; (T_a) denotes the time interval from the start of the surface runoff period to the start of the maximum runoff period.

When considering T_c as the dependent variable and the others as independent variables, an equation that relates all of these variables can be constructed. Therefore the T_c can be defined as function of h_L , h_{ef} , P_{abs} , Q_p , T_d , i_{avg} , i_{max} , T_{CG} , T_r , T_b , T_p and T_a . Thus, the independent variables influencing on the dependent variable can be determined using Pearson correlation coefficient, as a measure of dependencies between variables (Kohnová and Szolgay 2003; Latt et al. 2015).

2.4 Correlation Analysis

The Pearson linear correlation coefficient (r), shown in Eq. 2, describes the linear correlation between two random variables (x, y) and is not dependent on the measurement unit.

$$r = \frac{n \sum_{i=1}^n x_i y_i - \sum_{i=1}^n x_i \sum_{i=1}^n y_i}{\sqrt{\left[n \sum_{i=1}^n x_i^2 - \left(\sum_{i=1}^n x_i \right)^2 \right] \cdot \left[n \sum_{i=1}^n y_i^2 - \left(\sum_{i=1}^n y_i \right)^2 \right]}} \tag{2}$$

As this coefficient approaches 1 or -1, correlations in the observed data grow stronger. A correlation of zero denotes the absence of a linear relationship between variables (Dashtaki et al. 2009). Table 1 shows other degrees of correlation between variables.

Through correlation analysis, it is possible to evaluate the relationship between the independent variables and the dependent variable T_c . The relationship between the T_c and the independent variables was verified using a linear correlation matrix based on a 95 % confidence interval. The null hypothesis (H_0) was taken as the assertion that the dependent variable T_c can be influenced by the independent variables. Based on the matrix, the two variables with the highest correlation coefficients were used to define the model (Eq. 3).

$$T_c = K \cdot V_1^m \cdot V_2^n \tag{3}$$

Here, V_1 and V_2 are the selected variables, and $K, m,$ and n are the equation parameters to be optimized.

2.5 Harmony Search

As originally proposed by Geem et al. (2001), the harmony search (HS) algorithm was conceptualized from the musical process of searching for a ‘perfect state’ of harmony, such as jazz improvisation. The same authors claim that jazz improvisation seeks a best state determined by aesthetic estimation, just as the optimization algorithm seeks a best state determined by evaluating

Table 1 Degrees of correlation between variables according to the Pearson coefficient

Pearson correlation coefficient	Degree of correlation
$0.90 < r \leq 0.99$	Very strong
$0.70 < r \leq 0.90$	Strong
$0.30 < r \leq 0.70$	Moderate
$0 < r \leq 0.30$	Weak

the objective function. According to Kougiyas and Theodossiou (2013) it is a music inspired method, imitating the music creation process in order to find optimal solutions in complicated problems.

The HS algorithm has been applied in various optimization problems, including those involving structural design (Saka 2007), economic uses of heat and power (Vasebi et al. 2007), water network projects (Geem 2009), groundwater management schemes (Ayvaz 2009), geotechnical stability problems (Cheng et al. 2008), and hydrological model parameter estimations (Geem 2010).

The fit between the estimated and observed times was verified using the objective function F_{obj} , which seeks to minimize the sum of squared differences between observed and estimated values (Eq. 4).

$$F_{obj} = \sum_{i=1}^n (T_{C_{obs}} - T_{c_e})^2 \quad (4)$$

Here, n is the number of events, $T_{c_{obs}}$ is the time of concentration obtained from a graphical analysis of hyetographs and hydrographs, and T_{c_e} is the time of concentration estimated via the proposed model. The fit between the observed and estimated values was assumed when the objective function was held at a minimum value, i.e., when the sequence of iterations did not improve the quality of results obtained, thereby ceasing the optimization process.

The quality of fit between observed and estimated time values was calculated using the Nash-Sutcliffe efficiency coefficient (Nash and Sutcliffe 1970; Patel and Ramachandran 2015; Liu et al. 2016), as shown in Eq. 5.

$$NS = 1 - \frac{\sum_{i=1}^n (T_{C_{obs}} - T_{C_e})^2}{\sum_{i=1}^n (T_{C_{obs}} - T_{C_{obs}})^2} \quad (5)$$

Here, NS is the Nash-Sutcliffe efficiency coefficient; $T_{c_{obs}}$ is the time of concentration obtained from a graphical analysis of hyetographs and hydrographs; T_{c_e} is the time of concentration estimated using the model; and $T_{C_{obs}}$ is the average of all observed times of concentration.

The NS can range from negative infinity to 1.0. A negative sign denotes that the model exhibits a poor fit, suggesting that the difference between what was observed and estimated is even greater than the variance between observed values. When the value of NS reaches unity, this denotes a perfect fit between what was estimated and observed. Thus, a value that approaches unity denotes a more efficient and accurate model. Santhi et al. (2001) presented the following classification for this coefficient: when $NS > 0.65$, the model is deemed very good; when $0.54 < NS < 0.65$, the model is deemed good; and when $0.5 < NS < 0.54$, the model is deemed satisfactory. According to Gottschalk and Motovilov (2000), the performance of a model is considered suitable if the NS value exceeds 0.75 and is considered acceptable if this value ranges between 0.36 and 0.75.

Figure 2 presents a flowchart of the proposed model.

3 Results and Discussion

Figure 3 presents the hydrograph of one of the 17 selected rainfalls events with the T_c and the end of the surface runoff period marked.

Independent variables determined from the graphical analysis of the hyetographs and hydrographs for the selected events are shown in Table 2.

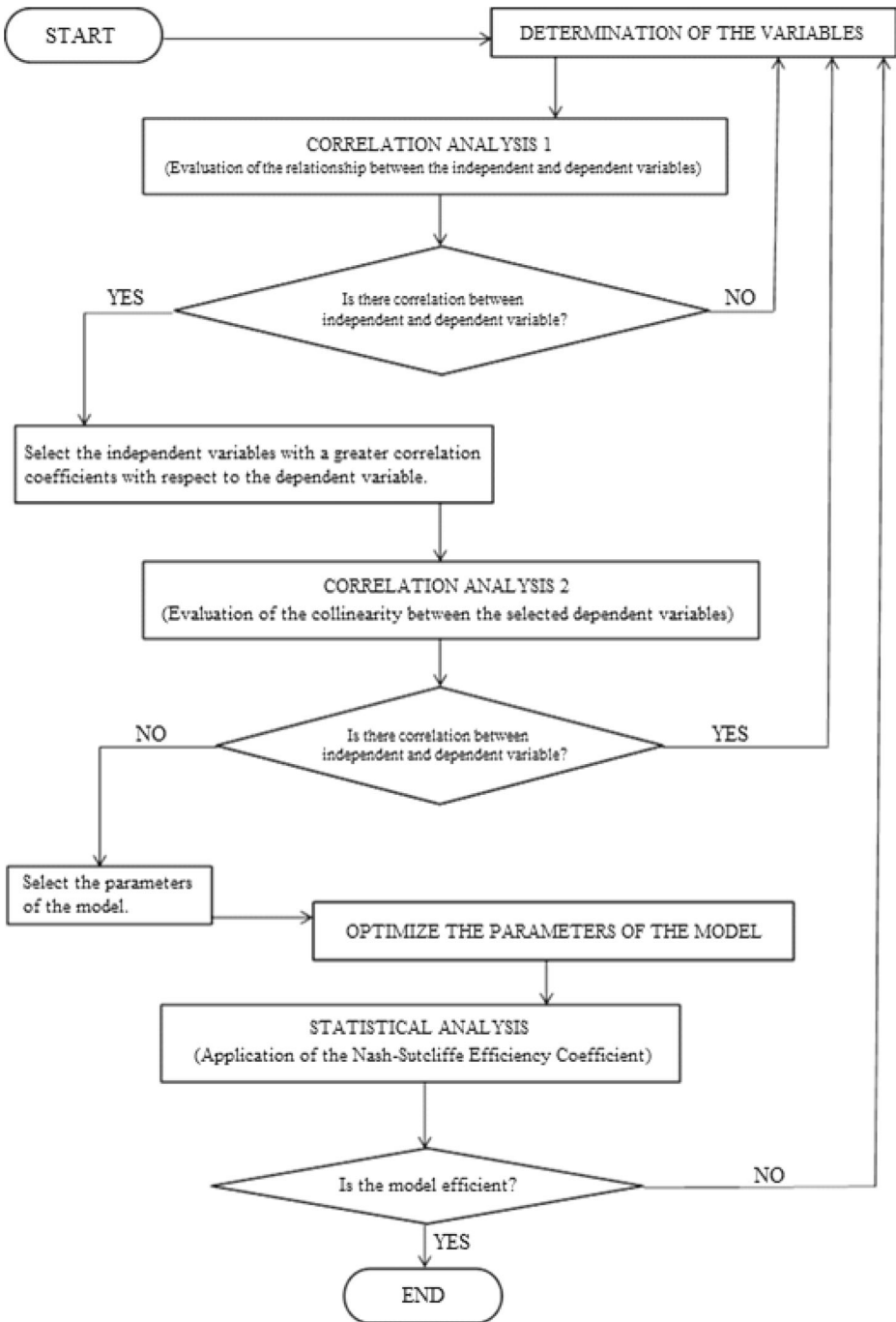


Fig. 2 Flowchart of the proposed model

Istok and Boersma (1986) studied the effect of previous rainfall events on surface runoff generation and found that relative to rainfall intensity, antecedent moisture has more significant

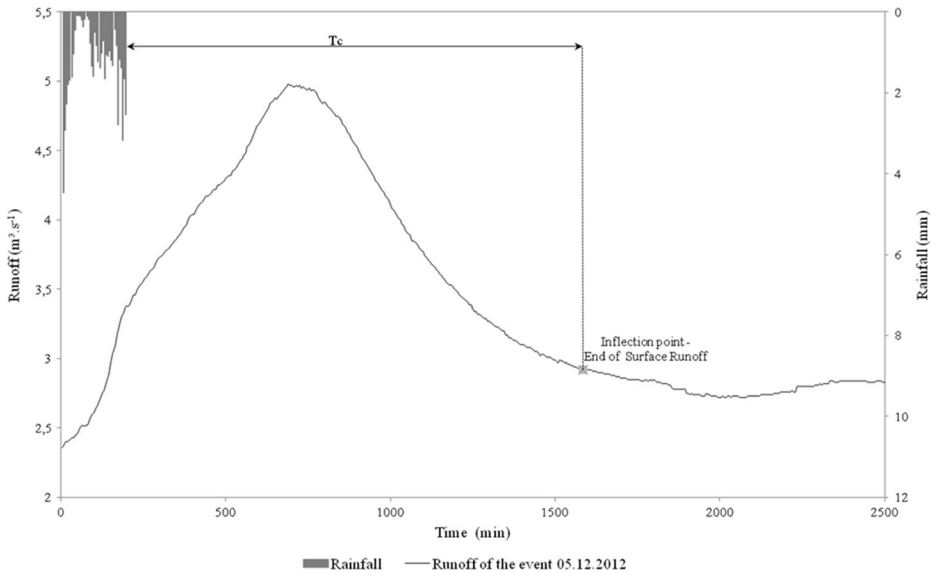


Fig. 3 Hydrograph of the 05/12/2012 event with the T_c and the end of the surface runoff period marked

effects on the volume of water drained. This result necessitated an analysis of P_{abs} and h_{ef} . The lowest value of P_{abs} (98.56 %) was observed on 11/11/13 (after no rainfall for the previous eight days) and the highest value (99.71 %) was observed on 2/28/13 (after no rainfall for the previous three days), and both were recorded during rainy seasons. The lowest value of h_{ef} (0.07 mm) was observed on 12/2/12 and the highest value of h_{ef} (0.68 mm) was observed on 5/12/12. It should be noted that these two events occurred following similar periods of zero precipitation (one day). Such observations show that in the study area, variations in time elapsed since preceding periods of rainfall do not influence surface runoff generation patterns. In the sub-watershed of the Guariroba stream, 0.74 % of total rains turn into direct surface runoff on average. It is understood that the high average value of P_{abs} (99.26 %) and low average value of h_{ef} (0.31 mm) can be attributed to the dominant class of soil present in the study area, as it Orthic Quartzarenic Neosol (RQo) is derived from sandstone rock and exhibits high infiltration capacities and low retention capacities for moisture.

To a certain extent, were found variations in T_c values (18,20 h – 26,39 h) obtained from the events analysis. As the rainfall-runoff process in a basin is intrinsically nonlinear, it is practically impossible to determine the unique value of time of concentration to all rainfall-runoff selected events. Thus, assigning a unique value for the T_c of a particular watershed generates an estimate of the average T_c value for the watershed. Thus, methodologies for estimating T_c generate this average value.

The analysis of the simple linear correlation between the independent variables and the T_c resulted in the development of a linear correlation matrix (Table 3). In this table, the top row includes coefficient values, which represent the degree and direction of correlations. Values shown in parentheses below each coefficient help identify whether a correlation significantly differs from zero. The smaller this value is, the greater the chance of a linear correlation, i.e., the higher the certainty that the null hypothesis is not rejected.

According to the results obtained by McCuen et al. (1984), the intensity of effective rainfall is highly correlated with T_c . However, in this study, no significant correlation between

Table 2 Variables determined from the graphical analysis of the hietographs and hydrographs

Item	Event	T _c (hours)	h _t (mm)	h _{ef} (mm)	P _{obs} (%)	Q _p (m ³ .s ⁻¹)	T _d (hours)	i _{avg} (mm.h ⁻¹)	i _{max} (mm/5 min)	T _{CG} (hours)	T _r (hours)	T _b (hours)	T _p (hours)	T _a (hours)
1	11/14/2011	26.18	63.25	0.41	99.35	3.46	8.49	7.45	8.01	5.08	14.65	34.02	14.94	19.37
2	02/01/2012	20.42	37.25	0.13	99.66	3.03	3.11	11.96	8.48	0.45	6.84	23.04	16.25	16.20
3	02/03/2012	24.19	31.50	0.30	99.05	3.75	5.51	5.71	6.49	4.30	14.70	29.12	10.70	14.42
4	03/13/2012	23.21	17.75	0.19	98.91	3.25	6.28	2.82	2.04	1.43	14.20	27.24	13.86	13.04
5	05/12/2012	22.32	49.50	0.68	98.63	4.98	4.01	12.35	4.47	1.70	14.83	26.24	9.80	11.42
6	09/21/2012	23.54	63.75	0.42	99.34	3.85	7.47	8.54	5.70	2.46	14.93	30.51	13.63	15.58
7	12/02/2012	18.20	14.50	0.07	99.51	2.74	2.63	5.51	4.69	1.98	8.42	20.75	10.44	12.33
8	12/28/2012	24.17	67.00	0.52	99.22	4.24	8.08	8.29	8.81	4.83	19.84	30.08	7.59	10.24
9	12/30/2012	21.13	27.50	0.16	99.40	3.26	3.86	7.12	5.54	1.13	13.42	24.75	10.45	11.33
10	02/28/2013	23.54	76.75	0.22	99.71	3.19	7.17	10.70	7.31	1.48	12.75	30.12	16.48	17.38
11	03/24/2013	23.50	68.50	0.39	99.43	3.85	2.67	25.67	8.36	0.71	14.17	26.00	11.29	11.83
12	04/07/2013	22.64	37.75	0.33	99.13	4.18	3.62	10.44	4.26	0.92	13.58	25.84	11.74	12.25
13	06/02/2013	24.00	47.25	0.22	99.54	3.42	5.17	9.14	5.26	2.12	10.92	28.67	16.13	17.75
14	07/01/2013	23.57	20.50	0.16	99.21	3.00	8.99	2.28	3.00	2.59	15.42	32.39	14.55	16.97
15	10/04/2013	23.41	40.50	0.31	99.24	3.32	9.18	4.41	4.33	3.79	12.42	32.09	16.38	19.67
16	10/22/2013	23.60	35.00	0.15	99.58	2.78	4.24	8.26	3.61	1.83	19.01	26.84	7.00	7.83
17	11/11/2013	26.39	37.75	0.54	98.56	4.01	5.55	6.80	8.25	2.81	15.30	31.86	13.84	16.57
	Sum	394.00	736.00	5.20	1687.48	60.31	96.04	147.46	98.61	39.60	235.38	479.56	215.08	244.18
	Average	23.18	43.29	0.31	99.26	3.55	5.65	8.67	5.80	2.33	13.85	28.21	12.65	14.36
	Standart deviation	1.94	18.96	0.17	0.33	0.59	2.24	5.25	2.11	1.42	3.18	3.58	3.04	3.40

Table 3 Simple linear correlation matrix (r -Pearson)

	T_c (hours)	h_t (mm)	h_{ef} (mm)	P_{abs} (%)	Q_p ($m^3 \cdot s^{-1}$)	T_d (hours)	i_{med} ($mm \cdot h^{-1}$)	i_{max} ($mm/5 \text{ min}$)	T_{CG} (hours)	T_r (hours)	T_b (hours)	T_p (hours)	T_a (hours)
T_c (hours)	1												
h_t (mm)	0.442 (0.075)	1											
h_{ef} (mm)	0.526 (0.030)	0.528 (0.029)	1										
P_{abs} (%)	-0.365 (0.155)	0.216 (0.404)	-0.670 (0.003)	1									
Q_p ($m^3 \cdot s^{-1}$)	0.325 (0.203)	0.417 (0.096)	0.915 (0.000)	-0.664 (0.004)	1								
T_d (hours)	0.575 (0.016)	0.248 (0.338)	0.186 (0.476)	-0.102 (0.698)	-0.028 (0.916)	1							
i_{med} ($mm \cdot h^{-1}$)	-0.028 (0.914)	0.582 (0.014)	0.280 (0.276)	0.211 (0.416)	0.342 (0.179)	-0.516 (0.034)	1						
i_{max} ($mm/5 \text{ min}$)	0.264 (0.306)	0.637 (0.006)	0.362 (0.154)	0.154 (0.554)	0.242 (0.349)	-0.047 (0.859)	0.498 (0.042)	1					
T_{CG} (hours)	0.550 (0.022)	0.192 (0.461)	0.362 (0.153)	-0.221 (0.393)	0.148 (0.570)	0.699 (0.002)	-0.428 (0.087)	0.232 (0.370)	1				
T_r (hours)	0.613 (0.009)	0.260 (0.313)	0.477 (0.053)	-0.349 (0.170)	0.363 (0.152)	0.389 (0.123)	-0.064 (0.807)	-0.036 (0.892)	0.438 (0.079)	1			
T_b (hours)	0.868 (0.000)	0.385 (0.127)	0.403 (0.109)	-0.254 (0.325)	0.163 (0.532)	0.880 (0.000)	-0.292 (0.256)	0.130 (0.620)	0.688 (0.002)	0.499 (0.041)	1		
T_p (hours)	0.165 (0.527)	0.104 (0.692)	-0.194 (0.456)	0.162 (0.533)	-0.261 (0.311)	0.372 (0.141)	-0.132 (0.613)	0.063 (0.809)	-0.057 (0.829)	-0.570 (0.017)	0.360 (0.155)	1	
T_a (hours)	-0.102 (0.696)	0.272 (0.291)	-0.039 (0.883)	0.299 (0.243)	-0.105 (0.689)	0.016 (0.953)	0.107 (0.682)	0.362 (0.153)	0.393 (0.118)	-0.024 (0.926)	-0.075 (0.775)	-0.075 (0.775)	1

Table 4 Initial uncertainty range of the variables to be optimized

Variable	L _{low}	L _{up}
K	2.00	4.00
m	0.40	0.60
n	0.05	0.25

characteristic parameters of event rainfall and T_c was found. This may be attributable to the fact that interception and infiltration processes in rural watersheds are largely shaped by sandy soil effects on water balance levels.

Given that the correlations with a significance level of 5 % are acceptable, five significant correlations for T_c were deemed distinct. A strong correlation was found between T_c and T_b (0.868), and moderate correlations were found between T_c and h_{ef} (0.526), T_d (0.515), T_{CG} (0.550), and T_r (0.613). The sign of the coefficient denotes the proportionality of the relationship. If both variables tend to increase or decrease simultaneously, the coefficient is positive. If a variable tends to increase as the other decreases, the coefficient is negative.

The Pearson correlation coefficient of the relationship between T_c and T_b is 0.868, and the *p*-value is close to zero, indicating that the observed effect is not attributable to chance. Thus, it shows that T_c tends to increase as T_b increases. A similar relationship is found for T_c and T_r.

In cases in which independent variables are strongly correlated, a problem of multicollinearity arises. Collinear variables affect the interpretation of model coefficients. To prevent cases of multicollinearity, Naghettini and Pinto (2007) recommend eliminating one independent variable for each set of two independent variables with a correlation coefficient greater than 0.85. The strongest correlations were found between T_c and T_b and between T_c and T_r. Thus, collinearity probabilities for T_b and T_r were examined. At a 4.1 % significance level, i.e., within a 95 % confidence interval, it was verified that the correlation coefficient between T_b and T_r is 0.499, proving the non-collinearity of the analyzed variables.

Thus, it is proposed that T_c is a function of T_b and T_r, i.e., T_c = f (T_b, T_r); based on this assumption, the model is formulated (Eq. 6).

$$T_c = K \cdot T_b^m \cdot T_r^n \tag{6}$$

Table 5 Final values generated for the variable based on a 10-value harmonic memory

K	m	n	F _{obj}
3.651	0.494	0.076	11.835
3.652	0.494	0.076	11.835
3.652	0.494	0.076	11.839
3.657	0.494	0.076	11.842
3.652	0.494	0.076	11.843
3.652	0.494	0.077	11.845
3.658	0.494	0.076	11.848
3.654	0.494	0.077	11.862
3.658	0.494	0.076	11.866
3.655	0.493	0.076	11.886
3.657	0.493	0.076	11.906

The model parameters were optimized using the HS algorithm. Initially, an uncertainty range with lower and upper limits for values of K , m , and n was stipulated (Table 4).

A routine that reads the limits established for each variable and that generates a random value for these limits was written based on a harmonic memory parameter, $NMH = 10$, for each of the three variables involved in the optimization process, where the number of iterations, $NIT = 100$. After each iteration, the 10 best ordered sets were selected, and the lower and upper limits of each variable were used in the next iteration. This method was repeated until the objective function reached a minimum value, i.e., when the sequence of iterations did not further improve the quality of results obtained (Table 5).

In turn, the model for estimating T_c was formulated (Eq. 7).

$$T_c = 3,651 T_b^{0,494} T_r^{0,076} \quad (7)$$

To determine the model's predictive capacity, the NS efficiency coefficient was calculated, and was found to be 0.804. These statistical analyses show that according to Gottschalk and Motovilov (2000), the performance of the model is appropriate and good, and according to Santhi et al. (2001) parameters, the model is deemed very good, achieving a Nash-Sutcliffe coefficient of 0.804.

4 Conclusion

Variations in periods elapsed since a previous rainfall period do not influence surface runoff generation patterns in watersheds similar the study area. The base time of the hydrograph and the time interval between the start of maximum runoff and the end of surface runoff constitute the main hydrological variables that affect the time of concentration in watersheds with features that reflect those of the study area. The proposed model allows one to estimate the time of concentration using only the hydrograph of an event at the base level of a watershed and without the use of any rainfall data. These results are promising and show that the applied methodology is efficient and effective in estimating time of concentration values for watersheds.

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Compliance with Ethical Standards

Conflict of Interest The authors declare that they have no conflict of interest.

References

- Ayvaz MT (2009) Application of harmony search algorithm to the solution of groundwater management models. *Adv Water Resour* 32(6):916–924. doi:10.1016/j.advwatres.2009.03.003
- Bondelid TR, McCuen RH, Jackson TJ (1982) Sensitivity of SCS models to curve number variation. *Water Resour Bull* 18(1):111–116. doi:10.1111/j.1752-1688.1982.tb04536.x

- Cheng YM, Li L, Lansivaara T, Chi SC, Sun YJ (2008) An improved harmony search minimization algorithm using different slip surface generation methods for slope stability analysis. *Eng Optim* 40(2):95–115. doi:10.1080/03052150701618153
- Chow VT, Maidment DR, Mays LW (1988) *Applied hydrology*. McGraw-Hill, New York, p. 572
- Dashtaki SG, Homaei M, Mahdian MH, Kouchakzadeh M (2009) Site-dependence performance of infiltration models. *Water Resour Manag* 23(13):2777–2790. doi:10.1007/s11269-009-9408-3
- Dykes AP, Thomes JB (2000) Hillslope hydrology in tropical rainforest steeplands in Brunei. *Hydrol Process* 14(2):215–235. doi:10.1002/(SICI)1099-1085(20000215)14:2<215::AID-HYP921>3.0.CO;2-P
- Eagleson PS (1970) *Dynamic hydrology*. McGraw-Hill, New York, p. 462
- Fang X, Thompson DB, Cleveland TG, Pradhan P, Malla R (2008) Time of concentration estimated using watershed parameters determined by automated and manual methods. *J Irrig Drain Eng* 134(2):202–211. doi:10.1061/(ASCE)0733-9437(2008)134:2(202)
- Geem ZW (2009) Particle-swarm harmony search for water network design. *Eng Optim* 41(4):297–311. doi:10.1080/03052150802449227
- Geem ZW (2010) Parameter estimation of the nonlinear Muskingum model using parameter-setting-free harmony search. *J Hydrol Eng* 16(8):684–688. doi:10.1061/(ASCE)HE.1943-5584.0000352
- Geem ZW, Kim JH, Loganathan GV (2001) A new heuristic optimization algorithm: harmony search. *Simulation* 76(2):60–68. doi:10.1177/003754970107600201
- Gottschalk L, Motovilov Y (2000) Macro-scale hydrological modeling – a Scandinavian experience. International symposium on: ‘can science and society save the water crisis in the twenty-first century – report from the world’ Japan Society of Hydrology and Water Resources. Tokyo 1:38–45
- Gwenzi W, Nyamadzawo G (2014) Hydrological impacts of urbanization and urban roof water harvesting in water-limited catchments: a review. *Environmental Processes*, 1:573–593. doi:10.1007/s40710-014-0037-3
- Haan CT, Barfield BJ, Hayes JC (1994) *Design hydrology and sedimentology for small catchments*. Academic Press, p. 588
- Istok JD, Boersma L (1986) Effect of antecedent rainfall on runoff during low-intensity rainfall. *J Hydrol* 88(3):329–342. doi:10.1016/0022-1694(86)90098-3
- Kang JH, Kayhanian M, Stenstrom MK (2008) Predicting the existence of stormwater first flush from the time of concentration. *Water Res* 42(1):220–228. doi:10.1016/j.watres.2007.07.001
- Kohnová S, Szolgay J (2003) Regional estimation of the index flood and the standard deviation of the summer floods in the Tatry mountains. *J Hydrol Hydromech* 51(4):241–255
- Konrad CP (2003) Effects of urban development on floods. U.S Geological survey fact sheet 076-03. <http://pubs.usgs.gov/fs/fs07603>. Accessed 4 July 2014
- Kougias IP, Theodossiou NP (2013) Multiobjective pump scheduling optimization using harmony search algorithm (HSA) and polyphonic HSA. *Water Resour Manag* 27(5):1249–1261. doi:10.1007/s11269-012-0236-5
- Kuichling E (1889) The relation between the rainfall and the discharge of sewers in populous areas. *Trans Am Soc Civ Eng* 20(1):1–56
- Latt ZZ, Wittenberg H, Urban B (2015) Clustering hydrological homogeneous regions and neural network based index flood estimation for ungauged catchments: an example of the Chindwin River in Myanmar. *Water Resour Manag* 29(3):913–928. doi:10.1007/s11269-014-0851-4
- Li MH, Chibber P (2008) Overland flow time of concentration on very flat terrains. *Transportation Research Record: Journal of the Transportation Research Board* 2060(1):133–140. doi:10.3141/2060-15
- Liang J, Melching CS (2012) Comparison of computed and experimentally assessed times of concentration for a V-shaped laboratory watershed. *J Hydrol Eng* 17(12):1389–1396. doi:10.1061/(ASCE)HE.1943-5584.0000609
- Liu Z, Guo S, Zhang H, Liu D, Yang G (2016) Comparative study of three updating procedures for real-time flood forecasting. *Water Resour Manag* 30(7):2111–2126. doi:10.1007/s11269-016-1275-0
- McCuen RH, Wong SL, Rawls WJ (1984) Estimating urban time of concentration. *J Hydraul Eng* 110(7):887–904. doi:10.1061/(ASCE)0733-9429(1984)110:7(887)
- Mulvany TJ (1851) On the use of self-registering rain and flood gauges in making observations of the relations of rainfall and flood discharges in a given catchment. *Proceedings of the Institution of Civil Engineers of Ireland* 4(2):18–33. doi:10.1061/(ASCE)0733-9429(1984)110:7(887)
- Naghetini M, Pinto EJA (2007) *Hidrologia Estatística*. Belo Horizonte, Serviço Geológico do Brasil – CPRM, p. 561
- Nash JE, Sutcliffe JV (1970) River flow forecasting through conceptual models, part I: a discussion of principles. *J Hydrol* 10(3):282–290. doi:10.1016/0022-1694(70)90255-6
- Ogbonna SU (2004) Formula for the time of concentration of runoff. *J Hydraul Eng* 130(6):576–579. doi:10.1061/(ASCE)0733-9429(2004)130:6(576)

- Patel SS, Ramachandran P (2015) A comparison of machine learning techniques for modeling river flow time series: the case of upper Cauvery river basin. *Water Resour Manag* 29(2):589–602. doi:[10.1007/s11269-014-0705-0](https://doi.org/10.1007/s11269-014-0705-0)
- Pavlovic SB, Moglen GE (2008) Discretization issues in travel time calculation. *J Hydrol Eng* 13(2):71–79. doi:[10.1061/\(ASCE\)1084-0699\(2008\)13:2\(71\)](https://doi.org/10.1061/(ASCE)1084-0699(2008)13:2(71))
- Saka MP (2007) Optimum geometry design of geodesic domes using harmony search algorithm. *Adv Struct Eng* 10(6):595–606. doi:[10.1260/136943307783571445](https://doi.org/10.1260/136943307783571445)
- Santhi C, Arnold JG, Williams JR, Dugas WA, Sirinivasan R, Hauck LM (2001) Validation of the SWAT model on a large river basin with point and nonpoint sources. *J Am Water Resour Assoc* 37(5):1169–1188. doi:[10.1111/j.1752-1688.2001.tb03630.x](https://doi.org/10.1111/j.1752-1688.2001.tb03630.x)
- Sharifi S, Hosseini SM (2011) Methodology for identifying the best equations for estimating the time of concentration of watersheds in a particular region. *J Irrig Drain Eng* 137(11):712–719. doi:[10.1061/\(ASCE\)IR.1943-4774.0000373](https://doi.org/10.1061/(ASCE)IR.1943-4774.0000373)
- Simas M J, Hawkins R H (2002) Lag time characteristics in small watersheds in the United States. Proc., 2nd Federal Interagency Hydrologic Modeling Conf., Las Vegas.
- Su D H, Fang X (2004) Estimating traveling time of flat terrain by 2-dimensional overland flow model. *Shallow flows*, G. Jirka and W. Uijtewaal, eds., Balkema, Rotterdam, The Netherlands, 629–635.
- Upegui JJV, Gutiérrez AB (2011) Estimación del tiempo de concentración y tiempo de rezago en la Cuenca experimental Urbana de La Quebrada San Luis, Manizales. *Dyna* 78(165):58–71
- Vasebi A, Fesanghary M, Bathaee SMT (2007) Combined heat and power economic dispatch by harmony search algorithm. *Int J Electr Power Energy Syst* 29(10):713–719. doi:[10.1016/j.ijepes.2007.06.006](https://doi.org/10.1016/j.ijepes.2007.06.006)
- Wong TSW (2005) Assessment of time of concentration formulas for overland flow. *J Irrig Drain Eng* 131(4):383–387. doi:[10.1061/\(ASCE\)0733-9437\(2005\)131:4\(383\)](https://doi.org/10.1061/(ASCE)0733-9437(2005)131:4(383))
- Wong TSW (2009) Evolution of kinematic wave time of concentration formulas for overland flow. *J Hydrol Eng* 14(7):739–744. doi:[10.1061/\(ASCE\)HE.1943-5584.0000043](https://doi.org/10.1061/(ASCE)HE.1943-5584.0000043)